

DEVELOPMENT VARIANCE PERMIT DVP #24-04

1. This Development Variance Permit (DVP) #24-04 (the "Permit"), issued pursuant to s. 498 of the *Local Government Act*, applies only to the following property:

Civic address: 2751 Production Way

Legal Description: PID: 000-593-915, Lot 313 District Lot 56 Group 1 NWP66423

(the "Lands")

- 2. This Permit varies the following sections of the *Burnaby Zoning Bylaw*, 1965, (the "**Zoning Bylaw**"), but only to the extent necessary to allow for the construction of an over height retaining wall substantially in accordance with what is outlined in bold clouding, and of varying heights along its length as shown, on the plans attached as Schedule A to this Permit:
 - Section 6.14.1 by increasing the permitted retaining wall height from 1.2 m to up to 9.5 m.
- 3. This Permit shall lapse if the owner does not substantially start any construction with respect to which this Permit is issued, within two (2) years of issuance of this Permit.
- 4. This Permit is not a building permit, sign permit, preliminary plan approval, or a subdivision approval.

AUTHORIZING RESOLUTION PASSED BY COUNCIL ON DAY OF , 20 THIS PERMIT IS HEREBY ISSUED THIS DAY OF , 20



4730 Kingsway, Burnaby, BC, Canada V5H 0C6

604-432-6200

Greater Vancouver Sewerage and Drainage District • Greater Vancouver Housing Corporation

www.metrovancouver.org

Schedule A - DVP# 24-04

GREATER VANCOUVER WATER DISTRICT

DEVELOPMENT VARIANCE PERMIT

IS NOT A BUILDING PERMIT

BUILDING PERMIT MUST BE APPLIED FOR

SEPARATELY

ISSUED FOR BUILDING PERMIT

TENDER NO. XX-XXXX

PRODUCTION WAY OPERATIONS CENTER RETAINING WALL AT 2751 PRODUCTION WAY

District File: W-XXXX

CITY OF BURNABY PLANNING DEPARTMENT

Schedule A

DVP#: 24-04

Address: 2751 Production Way

Variance:

Vary the permitted retaining wall height from 1.2m to 9.5m at the highest point.

NOVEMBER 2024



Burnaby BC Canada V5H 0C6

Tel: (604) 587-8400

www.stantec.com

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GENERAL NOTES:

- 1. ALL CONSTRUCTION AND MATERIALS SHALL BE IN ACCORDANCE WITH THE CONTRACT DOCUMENTS, DRAWINGS AND SPECIFICATIONS, CORPORATION STANDARDS, APPLICABLE MASTER MUNICIPAL CONSTRUCTION DOCUMENTS (MMCD), AND LOCAL, PROVINCIAL, AND FEDERAL REQUIREMENTS
- 2. MAINTAIN COPIES OF THE ABOVE DOCUMENTS ON SITE AND ENSURE THAT ALL TRADES ARE THOROUGHLY FAMILIAR WITH THE APPLICABLE SECTIONS OF THESE DOCUMENTS 3. ENSURE THAT ALL APPROVALS REQUIRED FOR THE PROPOSED WORKS HAVE BEEN OBTAINED FROM
- ALL AUTHORITIES AND AGENCIES PRIOR TO COMMENCEMENT OF ANY CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL PERMITS THAT MAY BE REQUIRED BY THE CITY OF BURNABY OR OTHER AGENCIES AND UTILITY COMPANIES FOR WORK.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SITE SAFETY FOR THE PROJECT. SITE SAFETY SHALL AT A MINIMUM BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE CORPORATION, WORKSAFE BC, AND THE CITY BURNABY. SITE SAFETY REQUIREMENTS SHALL ALSO BE IN ACCORDANCE WITH THE REQUIREMENTS OF UTILITY COMPANIES FOR WORK LOCATED ADJACENT TO OR NEAR THE UTILITY COMPANIES' WORKS. IN THE EVENT OF A CONFLICT OR DIFFERENCE IN STANDARDS BETWEEN SITE SAFETY REQUIREMENTS, THE MORE STRINGENT REQUIREMENT OR REQUIREMENTS SHALL APPLY.
- 5. THE LOCATIONS OF EXISTING SERVICES ARE BASED ON TOPOGRAPHIC SURVEY DATA AND RECORD DRAWINGS AND HAVE NOT BEEN VERIFIED. VERIFY THE LOCATION OF ALL THE EXISTING SERVICES PRIOR TO ANY CONSTRUCTION AND NOTIFY THE ENGINEER OF ANY DISCREPANCIES, CONFLICTS, OR
- 6. PROTECT, PRESERVE AND USE EXTREME CARE WHEN WORKING NEAR EXISTING SERVICES, UTILITIES, AND ABOVE AND BELOW GROUND FACILITIES, STRUCTURES, AND WORKS. THE CONTRACTOR SHALL REPAIR, REPLACE OR MAKE GOOD TO EXISTING CONDITION OR BETTER ANY DISTURBANCES TO THE SATISFACTION OF THE OWNER OF THE AFFECTED SERVICE OR UTILITY AND THE ENGINEER, AT THE CONTRACTOR'S OWN EXPENSE.
- 7. NOTIFY ALL APPROPRIATE PARTIES, THE CORPORATION AND ITS REPRESENTATIVE, CITY OF BURNABY, AND WHERE APPLICABLE, BC HYDRO, TELUS, FORTIS BC, AND THE CABLE COMPANY 48 HOURS PRIOR TO COMMENCING CONSTRUCTION. THE CONTRACTOR SHALL ALSO NOTIFY THE ENGINEER AT LEAST 48 HOURS IN ADVANCE OF COMMENCING CONSTRUCTION
- 8. PROVIDE WRITTEN NOTICE TO ALL PARTIES, BUSINESSES AND OFFICES, AFFECTED BY CONSTRUCTION OF THIS PROJECT AT LEAST 48 HOURS PRIOR TO CONSTRUCTION. COPIES OF THE NOTICE SHALL BE PROVIDED TO THE CORPORATION, THE ENGINEER AND THE CITY OF BURNABY
- 9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY COORDINATION THAT MAY BE REQUIRED WHEN WORKING ON OR NEAR CITY OF BURNABY WORKS AND FACILITIES AND UTILITY COMPANIES' FACILITIES. THE CONTRACTOR SHALL MAKE ALL NECESSARY ARRANGEMENTS, IF REQUIRED, FOR THE INSPECTION OF ALL REQUIRED SERVICE AND UTILITY CONNECTIONS.
- 10. SUMMARY 10.1. THE OWNER OF THE PROJECT IS METRO VANCOUVER.
- 10.2. THE OWNER'S REPRESENTATIVE IS STANTEC CONSULTING LTD. (STANTEC). TEL.: (604) 436-3014. FAX: (604) 436-3752. #500 - 4515 CENTRAL BLVD, BURNABY, BC V5H 0C6 (604) 436 - 3014
- GEOTECHNICAL ENGINEER: ADAM MCINTYRE, PENG adam.mcintyre@stantec.com
- STRUCTURAL ENGINEER: VISHWAS SHARMA, PENG
- vishwas.sharma@stantec.com
- CIVIL ENGINEER: CARTER CHAN, PENG
- carter.chan@stantec.com 10.3. THE PROJECT CONSISTS OF:
- 10.3.1. CLEARING AND GRUBBING OF VEGETATION.
- 10.3.2. REMOVAL OF THE EXISTING SURFICIAL FEATURES AND MATERIALS INCLUDING LANDSCAPING, ASPHALT AND CONCRETE, AND SITE INFRASTRUCTURE.
- 10.3.3. EXCAVATION AND REMOVAL OF ANY BURIED ABANDONED OR DECOMMISSIONED INFRASTRUCTURE AND MATERIALS (E.G., FOUNDATIONS, CONCRETE, BURIED UTILITY PIPES, MANHOLES, ETC.), RUBBLE, ORGANIC MATTER, AND ANY ORGANIC, SOFT, OR LOOSE SOILS.
- 10.3.4. BULK EXCAVATION TO DESIGN SUBGRADE.
- 10.3.5. CONSTRUCTION OF PERMANENT RETAINING WALLS:
 - 10.3.5.1. SOIL ANCHOR WALLS WITH TWO-STAGE FACING. 10.3.5.1.1. THE FIRST STAGE IS A TEMPORARY SHOTCRETE WALL WITH STEEL
 - WIRE MESH REINFORCEMENT THROUGHOUT THE WALL AND ADDITIONAL STEEL REINFORCING BARS AT EACH ANCHOR.
 - 10.3.5.1.2. THE SECOND STAGE CONSISTS OF A REINFORCED CAST-IN-PLACE (CIP) CONCRETE FACING, INCORPORATING THE ANCHOR HEAD
 - 10.3.5.1.3. INCLUDES A DRAINAGE SYSTEM ALONG THE FACE OF THE CUT SLOPE, A TOE DRAIN AT THE BASE OF THE CUT SLOPE AND IN FRONT OF THE REINFORCED CIP CONCRETE WALL FACING, AND AN INTERCEPTOR DITCH BEHIND THE TOP OF THE WALL
 - 10.3.5.2. INTERLOCKING CONCRETE RETAINING (ICR) WALLS. INCLUDES A LEVELLING PAD AT THE BASE OF THE BLOCKS, DRAINAGE ZONE BEHIND THE BLOCKS, DRAINAGE PIPE AT THE BOTTOM OF THE DRAINAGE ZONE, AND AN INTERCEPTOR DITCH BEHIND THE TOP OF THE WALL.
- 10.3.6. FINAL SITE GRADING 10.4. THE WORKS SHOWN ON THESE DRAWINGS SHALL BE BASED ON WRITTEN DIMENSIONS, AND
- 10.5. THE TERMS "SOIL ANCHORS" AND "ANCHORS" SHALL BE DEEMED TO MEAN PERMANENT GROUND ANCHORS.

<u>GENERAL NOTES - RETAINING WALLS</u>

- 1. DESIGN SPECIFICATIONS
- 1.1. METRO VANCOUVER ENGINEERING STANDARDS GEO-107 (2017), -109 (2012), AND -112
- 1.2. BRITISH COLUMBIA BUILDING CODE, BCBC (2024).
- 1.3. NATIONAL BUILDING CODE OF CANADA, NBCC (2020)
- 1.4. ENGINEERING AND GEOSCIENTISTS BRITISH COLUMBIA, EGBC, PROFESSIONAL PRACTICE GUIDELINES — RETAINING WALL DESIGN, VERSION 1.1 (2020).
- 1.5. CITY OF BURNABY ENGINEERING DEPARTMENT DESIGN CRITERIA MANUAL (2019). 1.6. MASTER MUNICIPAL CONSTRUCTION DOCUMENT, MMCD (2019).
- 1.7. CANADIAN FOUNDATION ENGINEERING MANUAL, CFEM, 51 EDITION (CANADIAN GEOTECHNICAL
- 1.8. PTI DC-35.1-14: RECOMMENDATIONS FOR PRESTRESSED ROCK AND SOIL ANCHORS, 5 IH EDITION (POST-TENSIONING INSTITUTE, 2014).
- 2. DESIGN PARAMETERS 2.1. SUBSURFACE CONDITIONS ARE ASSUMED TO BE AS SHOWN IN THE GEOTECHNICAL REPORT BY STANTEC CONSULTING LTD. (STANTEC) TITLED "RETAINING WALL AT 2751 PRODUCTION WAY, BURNABY, BC". REVISIONS TO THE DESIGN MIGHT BE REQUIRED WHERE DIFFERENT OR UNANTICIPATED CONDITIONS ARE ENCOUNTERED.
- 2.2. I = 1.0 SITE CLASS CPGA = 0.444 Sa(0.05) = 0.625 Sa(0.1) = 0.898 Sa(0.2) = 1.020 Sa(0.3) = 1.020 Sa(0.5) = 0.815Sa(1.0)=0.474 Sa(2.0)=0.287 Sa(5.0)=0.083
- 2.3. THE RETAINING WALLS SHOWN IN THESE DRAWINGS:
- 2.3.1. ARE NOT DESIGNED FOR HYDROSTATIC LOADING.
- 2.3.2. ACCOUNT FOR A TEMPORARY 50 KPA LIVE LOAD SURCHARGE ON THE GROUND SURFACE BEHIND THE 'NORTH WALL' (L-100), WITHIN THE BC HYDRO RIGHT-OF-WAY,

- TO ACCOUNT FOR FUTURE DUCT BANK CONSTRUCTION BY BC HYDRO.
- 2.4. GROUNDWATER CONTROL DURING CONSTRUCTION IS THE RESPONSIBILITY OF THE CONTRACTOR. IT IS ASSUMED THAT A DRAINAGE SYSTEM WILL BE INSTALLED TO REDUCE HYDROSTATIC PRESSURE ON THE WALLS.
- 3. GENERAL RESPONSIBILITIES OF THE CONTRACTOR 3.1. FURNISH ALL LABOUR, MATERIAL, SERVICES, AND EQUIPMENT NECESSARY FOR COMPLETION OF THE EXCAVATION SHORING, AS INDICATED ON THE DRAWINGS AND AS SPECIFIED HEREIN, INCLUDING BUT NOT LIMITED TO:
 - 3.1.1. BULK EXCAVATION OF THE SITE.
 - 3.1.2. REMOVAL OF EXCAVATED MATERIALS AND DEBRIS FROM THE SITE.
 - 3.1.3. PROVIDING AND MAINTAINING TEMPORARY ROADS AND ACCESS RAMP(S) WITHIN THE EXCAVATION AND REMOVAL OF TEMPORARY ACCESS RAMP(S) AS NECESSARY TO COMPLETE THE WORK SPECIFIED HEREIN.
- 3.2. FURNISHING OF MATERIAL, LABOUR, EQUIPMENT, AND ANY OTHER NECESSARY ITEMS FOR
- INSTALLATION, AND LOAD TESTING OF SOIL ANCHORS IN TENSION. 3.3. PERFORMANCE AND PROOF TESTS OF SOIL ANCHORS.
- 3.4. VISIT AND EXAMINE THE SITE AND NOTE ALL VISIBLE CHARACTERISTICS AND FEATURES AFFECTING THE WORK OF THIS SECTION. REPORT ANY UNSATISFACTORY CONDITIONS TO THE
- OWNER'S REPRESENTATIVE PRIOR TO COMMENCING THE WORK 3.5. CONTROL ALL DRILLING FLUIDS, WATER AND CUTTINGS DURING SOIL ANCHOR INSTALLATION CLEAN UP, AND OFF-SITE DISPOSAL OF ALL EXCESS FLUIDS, CUTTINGS AND REMOVAL OF ALL OTHER FOREIGN MATERIAL BROUGHT TO THE SITE DURING ANCHOR INSTALLATION AND
- 3.6. THE CONTRACTOR IS RESPONSIBLE FOR METHODS AND SEQUENCING OF CONSTRUCTION APPROPRIATE TO MAINTAIN EXCAVATION STABILITY DURING SHORING WORKS. EXCAVATION DIMENSIONS, ELEVATIONS, SETBACKS, AND CLEARANCES NOTED ON THESE DRAWINGS INDICATE
- 3.7. THE CONTRACTOR IS RESPONSIBLE FOR ALL SETTING OUT, INCLUDING LINES FOR CUTS, EXCAVATION GRADES, LOCATION OF ANCHORS, ETC. NO SLOPE SHALL BE CUT STEEPER THAN SHOWN ON THE DRAWINGS
- 3.8. NOTIFY THE OWNER'S REPRESENTATIVE AT LEAST FIVE WORKING DAYS BEFORE EXCAVATION IS TO START. THE OWNER'S REPRESENTATIVE SHALL HAVE ACCESS TO THE SITE WHENEVER REQUIRED. THE OWNER'S REPRESENTATIVE SHALL BE THE SOLE JUDGE OF THE EXTENT OF
- THE SITE REVIEW NECESSARY AND THE QUALIFICATIONS OF THE SITE REVIEWER. 3.9. THE CONTRACTOR SHALL NOT PROCEED WITH ANY STAGE OF EXCAVATION OR SHORING UNTIL IT HAS BEEN DISCUSSED AND CONFIRMED THAT THE CONTRACTOR'S PROPOSED METHODS AND SEQUENCING ARE IN GENERAL AGREEMENT WITH THE INTENT OF THE DESIGN.
- 3.10. PROTECT THE EXISTING INFRASTRUCTURE ADJACENT TO THE WORK AREA FROM DAMAGE BY CONTRACTOR'S WORK METHOD. ANY DAMAGE DONE OR SETTLEMENT CAUSED BY INADEQUATE MEASURES BEING TAKEN BY THE CONTACTOR TO PREVENT SAME, SHALL BE MADE GOOD IMMEDIATELY AT NO ADDITIONAL COST TO THE OWNER.
- 3.11. THE CONTRACTOR'S WORK SHALL COMPLY WITH ALL HEALTH, SAFETY, NOISE, AND SECURITY REQUIREMENTS OF WORKSAFE B.C., METRO VANCOUVER, THE CITY OF BURNABY, AND OTHER AUTHORITIES HAVING JURISDICTION.
- 4.1. SOIL ANCHOR WALLS
 - 4.1.1. THE CONTRACTOR SHALL SUBMIT TO THE OWNER'S REPRESENTATIVE FOR REVIEW AT LEAST SEVEN DAYS PRIOR TO THE START OF WORK, A DETAILED DESCRIPTION OF THE PROPOSED EXCAVATION SEQUENCING, ANCHOR CORROSON PROTECTION DETAILS, ANCHOR INSTALLATION AND TESTING PROCEDURE, INCLUDING DRILLING METHOD, GROUTING PROCEDURE, AND TESTING PROCEDURE
 - 4.1.2. THE CONTRACTOR SHALL SUBMIT A DETAILED DESCRIPTION OF THE EQUIPMENT TO BE USED FOR INSTALLING AND TESTING THE ANCHORS.
 - 4.1.3. THE CONTRACTOR SHALL ESTABLISH AND MAINTAIN A QUALITY CONTROL PROGRAM TO ENSURE COMPLIANCE WITH THE CONTRACT REQUIREMENTS, INCLUDING MAINTENANCE OF TEST RECORDS FOR ALL QUALITY CONTROL OPERATIONS. SUCH RECORDS SHALL BE PROVIDED TO THE OWNER'S ENGINEER FOR REVIEW ON REQUEST.
 - 4.1.4. PRIOR TO THE START OF THE WORK THE CONTRACTOR MUST SUBMIT THE FOLLOWING ITEMS TO THE OWNER'S REPRESENTATIVE:
 - 4.1.4.1. MILL TEST REPORTS FOR THE STEEL INCLUDING ANCHOR BARS, COUPLINGS, AND NUTS AND REINFORCEMENT.
 - 4.1.4.2. SHOP DRAWINGS OF THE ANCHOR ASSEMBLY.
 - 4.1.4.3. CALIBRATION RECORDS OF ALL JACKS, LOAD CELLS AND GAUGES TO BE USED FOR LOAD TESTING THE ANCHORS.
- 4.2. INTERLOCKING CONCRETE BLOCK (ICB) RETAINING WALLS
 - 4.2.1. PREPARE AND SUBMIT AN ICB RETAINING WALL REPORT CONSISTING OF ALL INFORMATION NECESSARY FOR THE FABRICATION, TRANSPORTATION, QUALITY CONTROL, QUALITY ASSURANCE AND INSTALLATION, INCLUDING BUT NOT LIMITED TO THE
 - 4.2.1.1. COMPLETE MATERIAL AND CONSTRUCTION SPECIFICATIONS. 4.2.1.2. DELIVERY DETAILS AND SCHEDULE.
 - 4.2.1.3. DETAILED BACKFILL PROCEDURES INCLUDING PROCEDURES FOR MONITORING DEFLECTIONS, ERECTION SAFETY, AND STRUCTURAL INTEGRITY DURING
 - 4.2.1.4. DETAILS FOR TEMPORARY CONSTRUCTION BRACINGS IF REQUIRED. 4.2.1.5. QUALITY CONTROL AND QUALITY ASSURANCE PROCEDURES.
- 4.3. DAILY RECORDS
- 4.3.1. FOR EACH INSTALLED ANCHOR THE CONTRACTOR SHALL KEEP DAILY RECORDS OF THE FOLLOWING:
 - 4.3.1.1. IDENTIFICATION NUMBER AND LOCATION OF THE ANCHOR.
 - 4.3.1.2. SEQUENCE OF INSTALLATION. 4.3.1.3. DATE OF INSTALLATION AND START AND FINISH TIME OF THE ANCHOR.
 - 4.3.1.4. DEPTH OF DRILLING AND APPROXIMATE SOIL TYPES ENCOUNTERED (FOR 4.3.1.5. GROUT PRESSURES ACHIEVED AND GROUT TAKE AT EACH INCREMENT OF
- GROUTING FOR ANCHORS. 5. PRODUCTS
- 5.1. SOIL ANCHOR WALLS

STRUCTURAL

Professional Seal

- 5.1.1. ANCHORS SHALL BE 517/690 MPA THREADBARS, MEETING ASTM A615.
- 5.1.2. ANCHORS SHALL BE DOUBLE CORROSION-PROTECTED (DCP) ANCHORS (CLASS I PROTECTED ANCHORS) IN ACCORDANCE WITH PTI DC 35.1-14.
- 5.1.3. ANCHOR INCLINATION, LENGTH. AND YIELD AND ULTIMATE STRENGTHS SHALL BE AS SHOWN IN THE DRAWINGS. NO DEVIATION IS PERMITTED WITHOUT WRITTEN APPROVAL BY THE OWNER'S REPRESENTATIVE.
- 5.1.4. PER BC HYDRO'S REQUIREMENTS, CASING SHALL BE USED WHEN DRILLING FOR ALL ANCHORS IN THE TOP ROW ALONG THE 'NORTH WALL' (L100). THE CASING SHALL NOT HAVE AN INSIDE DIAMETER LESS THAN 150 MM.
- 5.1.5. ANCHORS OTHER THAN THOSE IN THE TOP ROW ALONG THE 'NORTH WALL' (L100) AND ALL ANCHORS FOR THE 'SOUTH WALL' (L200) SHALL BE INSTALLED IN MINIMUM 150 MM DIAMETER HOLES, MINIMUM 300 MM LONGER THAN ANCHOR LENGTH. 5.1.5.1. ANCHOR CAPACITY IS DEPENDENT UPON THE INSTALLATION TECHNIQUES AND
 - TECHNIQUE AND EQUIPMENT TO ENSURE THE REQUIRED ANCHOR CAPACITY IS ACHIEVED. 5.1.5.2. IF CASING IS USED, THE CASING SHALL NOT HAVE AN INSIDE DIAMETER LESS

CIVIL

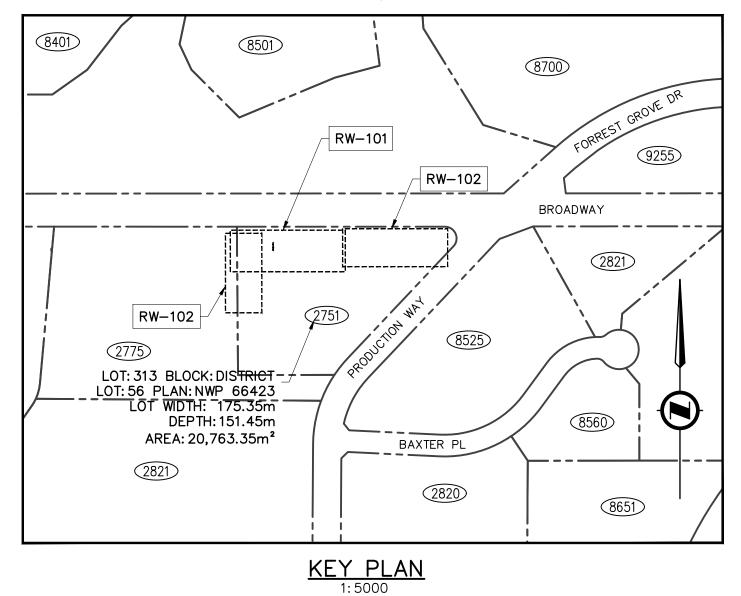
Professional Seal

EQUIPMENT, AND THE CONTRACTOR SHALL DETERMINE THE SUITABLE

THAN 150 MM. 5.1.6. THOROUGHLY FLUSH AND CLEAN-OUT THE HOLES USING COMPRESSED AIR OR AIR/WATER MIXTURE.



LOCATION PLAN



	DRAWING INDEX					
DISTRICT FILE No.	DWG. NO.	TITLE				
		PROJECT DRAWINGS				
W-XXXX	RW-000	COVER SHEET				
W-XXXX	RW-100	LOCATION PLAN, GENERAL NOTES, AND INDEX TO DRAWINGS				
W-XXXX	RW-101	GENERAL NOTES				
W-XXXX	RW-102	PLAN & ELEVATION				
W-XXXX	RW-103	PLAN & ELEVATION				
W-XXXX	RW-104	PLAN & ELEVATION				
W-XXXX	RW-105	SECTIONS				
W-XXXX	RW-106	SECTIONS				
W-XXXX	RW-107	DETAILS				



500 - 4515 Central Blvd Burnaby, BC V5H 0C6

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GEOTECHNICAL Professional Seal

							GREAT	ER VANCOUVER SEWERAGE AND DRAINAGE D	ISTRICT
							Design: MY	RETAINING WALL AT 2751 PRODUCTION WAY	SCALE:
Р3	300CT24	GH	JT	AM	VS	ISSUED FOR PERMIT	l	BURNABY, BC	NTS
P2	24MAY24	GH	JT	AM	VS	100% DETAILED DESIGN SUBMISSION			DISTRICT FILE W-XXXX
P1	19MAR24	GH	JT	AM	VS	90% DETAILED DESIGN SUBMISSION; DISTANCE FROM	Checked: AM	4	W-^^^
						RETAINING WALL TO BUILDING REDUCED FROM 9m TO 7.5m;	VS Approved		SHEET 1 OF 8
						ADDED PROPOSED BC HYDRO DUCT BANK BEHIND RETAINING WALL	SG	GENERAL NOTES, KEY PLAN & LOCATION PLAN	DRAWING NUMBER
Issue	Date	Deśn	Dr'n	Chkd	Appd	Description	Manager		RW-100
	_						I		

Bar is 20mm On Original Drawing. If Not On This Sheet, Adjust Scales Accordingly. | SUPERSEDES PRINTS OF THIS DRAWING NUMBER WITH LETTERS PREVIOUS TO ➡

<u>GENERAL NOTES - RETAINING WALLS (CONTINUATION)</u>

- 5.1.7. CENTRALIZERS SHALL BE PROVIDED FOR ANCHORS AT MAXIMUM 3 M SPACING AND BE CAPABLE OF POSITIONING THE ANCHOR IN THE DRILL HOLE SUCH THAT THE CONCENTRIC POSITION OF THE ANCHOR IS MAINTAINED ALONG ITS FULL LENGTH.
- 5.1.8. A BOND BREAKER, ACCEPTABLE TO THE OWNER'S REPRESENTATIVE, SHALL BE INSTALLED OVER THE FREE STRESSING LENGTH OF THE ANCHOR.
- 5.1.9. ANCHORS SHALL BE GROUTED USING "MICROSIL ANCHOR GROUT", WITH MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 40 MPA. THE CONTRACTOR IS RESPONSIBLE FOR MIX DESIGN AND TESTING OF THE GROUT.
- 5.1.10.ANCHORS SHALL BE COMPLETE WITH MANUFACTURER'S STANDARD ANCHOR PLATE, NUT AND ASSOCIATED HARDWARE. ANCHOR COMPONENTS INCLUDING COLLAR NUTS, ANCHOR PLATES, COUPLERS, CENTRALIZERS, O-RINGS, PVC SHEATHING, AND END CAPS ARE TO COMPLY WITH MANUFACTURER SPECIFICATIONS FOR DCP ANCHORS.
- 5.1.11. SHOTCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 30 MPA AT SEVEN DAYS AND 40 MPA AT 28 DAYS, AND A MINIMUM FLEXURAL STRENGTH OF 4 MPA AT SEVEN DAYS. THE CONTRACTOR IS RESPONSIBLE FOR MIX DESIGN AND TESTING OF THE SHOTCRETE.
- 5.1.12. SHOTCRETE SHALL BE REINFORCED WITH ONE LAYER OF 102 MM X 102 MM MW26 X MW26 WELDED WIRE MESH MEETING ASTM A1064.
- 5.1.13. ANCHOR HEAD REINFORCEMENT SHALL INCLUDE TWO 500 MM LONG REINFORCING STEEL BARS AT 200 MM SPACING, BOTH HORIZONTALLY AND VERTICALLY, CONFIRMING TO CSA
- 5.1.13.1. SIZE OF REINFORCING STEEL BARS VARIES AS SHOWN IN THE DRAWINGS. 5.1.13.2. REINFORCING STEEL BARS ARE ADDITIONAL TO THE WELDED WIRE MESH.
- 5.1.14.ANCHOR HEAD BEARING PLATE SHALL MEET THE REQUIREMENTS OF ASTM A36/44W GRADE STEEL. MINIMUM PLATE DIMENSIONS VARY AS SHOWN IN THE DRAWINGS.
- 5.2. ICB RETAINING WALLS
 - 5.2.1. ICB RETAINING WALL UNITS MEETING THE FOLLOWING REQUIREMENTS: 5.2.1.1. NEW, PRECAST CONCRETE BLOCKS WITH MINIMUM COMPRESSIVE STRENGTH OF 20 MPA AT 28 DAYS. PROVIDE TEST REPORTS CONFIRMING COMPRESSIVE STRENGTH TO ASTM C39. PREVIOULSY USED BLOCKS SHALL NOT BE USED.
 - 5.2.1.2. CONCRETE GREY FACE COLOUR AND SMOOTH FINISH. 5.2.1.3. NOMINAL DIMENSIONS OF 0.75 M WIDE X 0.75 M HIGH X 1.50 M LONG. 5.2.1.3.1. TO ACCOUNT FOR VARIATION IN WALL HEIGHT, UNITS WITH THE
 - FOLLOWING NOMINAL DIMENSIONS ARE PERMISSIBLE TO FINISH THE TOP OF THE WALL ONLY: 0.75 M WIDE X 0.375 M HIGH X 1.50 M
 - 5.2.1.4. ADEQUATE SHEAR KEYS ON THE TOP AND SUITABLE RECESSES ON THE BOTTOM SURFACES.
 - 5.2.1.5. MATCHING SHEAR KEYS AND RECESSES FOR INDIVIDUAL BLOCKS ALONG A PARTICULAR WALL.
 - 5.2.1.6. PROVIDED WITH RECESSED LIFTING LOOPS OR INSERTS; PROJECTING LIFTING LOOPS ARE NOT PERMITTED.
 - 5.2.1.7. EACH BLOCK CONSISTS OF A SINGLE MONOLITHIC POUR.
 - 5.2.1.8. NO EVIDENCE OF SURFACE SPALLING.
 - 5.2.1.9. EXTERIOR BLOCK DIMENSIONS UNIFORM AND CONSISTENT. MAXIMUM DIMENSIONAL DEVIATIONS 1% EXCLUDING THE ARCHITECTURAL SURFACE.
 - 5.2.1.10. FREE OF CHIPS AND CRACKS OR OTHER IMPERFECTIONS WHEN VIEWED FROM 3M UNDER DIFFUSED LIGHTING.
 - 5.2.2. GRANULAR BASE: 19 MM CRUSHED GRAVEL CONFORMING TO SECTION 31 05 17 OF THE MASTER MUNICIPAL CONSTRUCTION DOCUMENT, MMCD (2019).
 - 5.2.3. FREE DRAINING BACKFILL: 19 MM CLEAR CRUSHED GRAVEL. 5.2.4. STRUCTURAL BACKFILL: PIT RUN GRAVEL, PIT RUN SAND, OR RIVER SAND CONFORMING TO SECTION 31 05 17 OF MMCD (2019).
 - 5.2.5. GEOTEXTILE: NON-WOVEN GEOTEXTILE WITH A 'GRAB TENSILE STRENGTH' GREATER THAN 710 N AND 'APPARENT OPENING SIZE' GREATER THAN 0.2 MM.

EXECUTION 6.1. SOIL ANCHOR WALLS

- 6.1.1. FOR SHOTCRETE WORK ITEMS NOT COVERED HEREIN REFER TO THE LATEST VERSION OF
- ACI 506R GUIDE TO SHOTCRETE. 6.1.2. ANCHORS AND SHOTCRETE SHALL BE INSTALLED IN SEQUENCES AND STAGES TO MAINTAIN STABILITY OF THE EXCAVATION. THE EXCAVATION SHALL BE CARRIED OUT IN STAGES OF ALTERNATING PANELS.
 - 6.1.2.1. THE PANEL WIDTH SHALL NOT BE GREATER THAN 2 M FOR CUTS IN SOIL. 6.1.2.2. THE MAXIMUM DEPTH OF EXCAVATION FOR THE PANELS SHALL BE LIMITED TO
- 1.5 M. 6.1.2.3. THE ALTERNATING PANELS SHALL BE CUT WITH A SLOPE OR BERM TO
- MAINTAIN STABILITY, FOLLOWED BY ANCHOR INSTALLATION. 6.1.2.4. NO ADJACENT PANEL SHALL BE EXCAVATED CONCURRENTLY, AND NO MORE
- THAN ONE-THIRD OF THE TOTAL PANELS SHALL BE EXCAVATED CONCURRENTLY.
- 6.1.2.5. THE CONTRACTOR SHOULD NOT DEVIATE FROM THE PROPOSED EXCAVATION SEQUENCING. ANCHOR INSTALLATION AND TESTING PROCEDURE. AS OUTLINED IN CLAUSE 4.1, WITHOUT REVIEW BY THE OWNER'S REPRESENTATIVE.
- 6.1.3. ANCHOR GROUT: 6.1.3.1. GROUT SHALL BE INTRODUCED TO THE BOTTOM OF THE DRILL HOLE THROUGH A TREMIE TUBE IN A MANNER INTENDED TO FILL ALL VOIDS AND DISPLACE ANY WATER, SLOUGH, AND DEBRIS. IF THE TOTAL GROUT VOLUME CONSUMED IN ANY HOLE EXCEEDS THREE TIMES THE THEORETICAL VOLUME OF THE DRILL HOLE, WORK SHALL BE STOPPED AND THE OWNER'S REPRESENTATIVE SHALL BE NOTIFIED IMMEDIATELY.
 - 6.1.3.2. PREPARE A MINIMUM OF FIVE SETS OF FIVE GROUT CUBES THROUGHOUT THE GROUTING WORK. SETS OF GROUT CUBES SHALL BE COLLECTED AT EQUAL INTERVALS THROUGHOUT THE GROUTING WORK SCHEDULE. COMPRESSIVE TEST THE GROUT CUBES AT 1, 3, 7 AND 28 DAYS AND PROVIDE THE TEST RESULTS TO THE OWNER'S REPRESENTATIVE. SAMPLING AND TESTING SHALL BE IN CONFORMANCE WITH CSA A23.2.
- 6.1.3.3. PROVIDE DENSITY AND VISCOSITY OF THE GROUT SAMPLES. 6.1.4. THE SOIL SLOPE OR BERM OF A PANEL SHALL BE CUT NEATLY AFTER THE ANCHOR GROUT HAS GAINED A MINIMUM 20 MPA COMPRESSIVE STRENGTH.
 - 6.1.4.1. IMMEDIATELY AFTER THE CUT INSTALL THE PREFABRICATED VERTICAL GEO-COMPOSITE DRAIN, WELDED WIRE MESH, WITH ADDITIONAL REINFORCING STEEL BARS AROUND THE ANCHORS, AND SHOTCRETE THE CUT PANEL
 - 6.1.4.2. THE PREFABRICATED VERTICAL DRAIN SHALL BE CONTINUOUS FROM THE TOP TO THE BOTTOM OF THE CUT AND INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S REQUIREMENTS AND RECOMMENDATIONS.
- 6.1.5. WELDED WIRE MESH SHALL BE INSTALLED OVER THE ENTIRE CUT AND PLACED TO PROVIDE THE REQUIRED COVER ON BOTH SIDES OF THE MESH. MESH JOINTS SHALL BE LAPPED A MINIMUM 200 MM IN EACH DIRECTION (HORIZONTALLY AND VERTICALLY) AND TIED AT MINIMUM 100 MM INTERVALS WITH 16-GAUGE TIE WIRE TO ACHIEVE FULL
- CONTACT BETWEEN MESH LAYERS. 6.1.6. SHOTCRETE SHALL BE APPLIED IN ACCORDANCE WITH THE CURRENT AMERICAN CONCRETE INSTITUTE (ACI) DOCUMENTS "SPECIFICATION FOR SHOTCRETE" (ACI 506.2) AND "SHOTCRETE-GUIDE" (ACI PRC-506).
 - 6.1.6.1. SHOTCRETE SHALL BE MINIMUM 125 MM THICK. 6.1.6.2. SLEEVES OR PACKING SHALL BE USED DURING SHOTCRETE APPLICATION TO PREVENT CONTACT BETWEEN THE SHOTCRETE AND THE ANCHORS.
- 6.1.7. SHOTCRETE TEST PANELS (500 MM X 500 MM X 125 MM THICK) SHALL BE PREPARED BY THE CONTRACTOR.
 - 6.1.7.1. A MINIMUM OF 5 TEST PANELS SHALL BE PREPARED FOR THE PROJECT, AT

- EQUAL INTERVALS THROUGHOUT THE SHOTCRETE WORK SCHEDULE, AND GUNNED IN THE SAME POSITION AS THE WORK REPRESENTED.
- 6.1.7.2. A QUALIFIED TESTING AGENCY RETAINED BY THE CONTRACTOR SHALL RETRIEVE MINIMUM 4 CORES FROM EACH PANEL FOR BOTH COMPRESSIVE AND FLEXURAL STRENGTH DETERMINATION AT 24 HOURS (1 CORE), 3 DAYS (1 CORE) AND 28 DAYS (2 CORES). TEST RESULTS SHALL BE SUBMITTED TO THE OWNER'S REPRESENTATIVE.
- 6.1.8. EXCAVATION SHALL NOT PROCEED BELOW OR BESIDE APPLIED SHOTCRETE UNTIL: 6.1.8.1. THE COMPRESSIVE STRENGTH OF THE SHOTCRETE EXCEEDS 20 MPA. TESTS ON SHOTCRETE SAMPLES SHALL BE CARRIED OUT IN ACCORDANCE WITH ASTM C42, USING SPECIMENS CUT FROM 100 MM THICK TEST PANELS FORMED ON SITE IN ACCORDANCE WITH ACI PRC-506.
 - 6.1.8.2. THE ANCHORS HAVE BEEN SUCCESSFULLY TESTED. THE EXCAVATION FOR THE NEXT ROW OF ANCHORS SHALL NOT COMMENCE UNTIL THE TEST RESULTS HAVE BEEN REVIEWED AND ACCEPTED BY THE OWNER'S REPRESENTATIVE.
- 6.1.9. CONSTRUCT A SERIES OF 75 MM DIAMETER SCHEDULE 40 PVC CONNECTOR PIPES AT 3 M HORIZONTAL SPACING ALONG THE BASE OF THE WALL, ABOVE THE BOTTOM OF THE EXCAVATION, TO PERMIT FREE DRAINAGE OF WATER FROM THE GEO-COMPOSITE
- DRAINAGE SYSTEM, THROUGH THE TWO-STAGE WALL, TO A TOE DRAIN. 6.1.10. CONDUCT PERFORMANCE TESTS ON 10% OF THE ANCHORS IN CONFORMANCE WITH THE POST-TENSIONING INSTITUTE (PTI) MANUAL. CONDUCT PROOF TESTS ON ALL OTHER
 - 6.1.10.1. THE OWNER'S REPRESENTATIVE SHALL BE NOTIFIED OF THE TEST SCHEDULE IN ORDER THAT REVIEW OF THE TEST CAN BE CARRIED OUT.
 - 6.1.10.2. THE OWNER'S REPRESENTATIVE SHALL BE NOTIFIED IMMEDIATELY OF ANY ANCHOR NOT CAPABLE OF SUPPORTING THE TEST LOAD. ANCHORS NOT CAPABLE OF SUPPORTING THE TEST LOAD ARE TO BE REPLACED AT THE CONTRACTOR'S EXPENSE.
- 6.1.10.3. FOLLOWING SUCCESSFUL COMPLETION OF THE TEST, TENSION EACH ANCHOR TO THE DESIGN LOAD SHOWN IN THE DRAWINGS AND LOCK-OFF.
- 6.2. ICB RETAINING WALLS 6.2.1. STRIP ALL LOOSE OR ORGANIC SOIL FROM BENEATH THE BASE OF THE WALL AND FROM
 - 6.2.2. BENCH EXCAVATION TO SOUND MATERIAL FOR THE BASE OF THE WALL SYSTEM.
- 6.2.3. EXCAVATE TO THE DIMENSION SHOWN ON THE DRAWINGS OR AS DIRECTED BY THE OWNER'S REPRESENTATIVE.
- 6.2.4. CONSTRUCT THE WALLS TO MEET THE FOLLOWING REQUIREMENTS: 6.2.4.1. HORIZONTAL DIMENSIONS: \pm 25 MM.
 - 6.2.4.2. ELEVATIONS: \pm 25 MM.
 - 6.2.4.3. SLOPE: 1H:10V TOWARDS THE CUT SLOPE. 6.2.4.4. NO ABRUPT DEVIATIONS ARE VISIBLE.
- 6.2.5. LEVELLING PAD:
 - 6.2.5.1. PLACE 150 MM THICK LAYER OF GRANULAR BASE BELOW THE BOTTOM AND 150 MM BEYOND THE ICB RETAINING WALLS ON ALL SIDES.
 - 6.2.5.2. COMPACT TO A MINIMUM DRY DENSITY OF 95 PERCENT MODIFIED PROCTOR CORRECTED MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D1557 AND ASTM D4718.
- 6.2.6. DO NOT PROCEED WITH CONSTRUCTION WITHOUT PRIOR INSPECTION AND APPROVAL BY THE ENGINEER.
- 6.2.7. CARRY OUT CONSTRUCTION IN ACCORDANCE WITH THE CONTRACTOR'S QUALITY CONTROL AND QUALITY ASSURANCE AND CONSTRUCTION MONITORING PROGRAM AND ALL OTHER CONTRACT REQUIREMENTS.
- 6.2.8. INTERLOCK ALL ICB'S ABOVE THE BOTTOM COURSE WITH THE LOWER COURSE.
- 6.2.9. VERTICAL JOINTS SHALL BE STAGGERED WITH EACH SUCCESSIVE COURSE.
- 6.2.10. FREE DRAINING BACKFILL: 6.2.10.1. PLACEMENT OF FREE DRAINING BACKFILL TO CLOSELY FOLLOW ERECTION OF SUCCESSIVE COURSES OF INTERLOCKING CONCRETE BLOCKS.
 - 6.2.10.2. INSTALL A 150 MM DIAMETER PERFORATED PVC DRAINAGE PIPE AT THE BOTTOM OF THE FREE DRAINING BACKFILL. SLOPE THE DRAINAGE PIPE TO DRAIN WATER INTO A SUITABLE OUTLET.
 - 6.2.10.3. SEPARATE FREE DRAINING BACKFILL AND THE EXCAVATION FACE WITH GEOTEXTILE AS SHOWN ON THE DRAWINGS.
 - 6.2.10.4. AT NO TIME ALLOW THE DIFFERENCE IN ELEVATION BETWEEN THE FREE DRAINING BACKFILL AND THE TOP OF THE LAST ERECTED COURSE TO EXCEED THE HEIGHT OF ONE COURSE OF INTERLOCKING CONCRETE BLOCKS.
- 6.2.11. STRUCTURAL BACKFILL 6.2.11.1. STRUCTURAL BACKFILL IS DEFINED AS THE MATERIAL PLACED BETWEEN THE EXCAVATION FACE AND THE FREE DRAINING BACKFILL (IF ANY).
 - 6.2.11.2. SEPARATE FREE DRAINING BACKFILL AND STRUCTURAL BACKFILL WITH GEOTEXTILE AS SHOWN ON THE DRAWINGS
 - 6.2.11.3. PLACE STRUCTURAL BACKFILL IN MAXIMUM 300 MM THICK LIFTS AND COMPACT TO A MINIMUM DRY DENSITY OF 95 PERCENT MODIFIED PROCTOR CORRECTED
- MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D1557 AND ASTM D4718. 6.2.12. CONSTRUCT THE HANDRAIL, INTERCEPTOR DITCH, AND OTHER ASSOCIATED ITEMS AS SHOWN IN THE DRAWINGS.

GENERAL NOTES - UTILITIES RELOCATION AND PROTECTION

- 1. COMPLETE THE TERMINATION, CAPPING, RELOCATION, REQUIRED TESTING AND INSPECTION OF ALL TEMPORARILY OR PERMANENTLY RELOCATED UTILITIES AS SHOWN ON THE DRAWINGS PRIOR TO DECOMMISSIONING, DISCONNECTING, CUTTING, AND REMOVING ANY UTILITY LINE. OBTAIN WRITTEN
- APPROVAL FROM THE CORPORATION ON THE INSTALLATION OF THE UTILITIES. 2. NOTIFY CORPORATION AT LEAST 72 HOURS PRIOR TO DISCONNECTING, REMOVING, CUTTING, OR DECOMMISSIONING ANY UTILITY AND CLOSING OR OPENING ANY CONTROL VALVES.
- 3. IT IS CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT ALL POWER AND FLUID FLOWS ARE TERMINATED PRIOR TO RELOCATING, CUTTING, OR OPENING ANY UTILITY LINE. 4. ALL UTILITIES SHOWN TO BE REMOVED SHALL BE DISPOSED OF OFF-SITE AT A PERMITTED DISPOSAL FACILITY. NO MATERIAL SHALL BE STORED ON-SITE UNLESS SPECIFIED ON THE
- CONTRACT DOCUMENTS. 5. VERIFY THE LIMITS OF DEMOLITION WITH CORPORATION PRIOR TO STARTING DEMOLITION OF ANY
- STRUCTURE OR UTILITY. 6. VERIFY THAT ALL UTILITIES WITHIN ANY STRUCTURE, CHASE, OR ANY OTHER EQUIPMENT HAS BEEN DISCONNECTED AT THE SOURCE OR RE-ROUTED AS REQUIRED IN ACCORDANCE WITH THIS CONTRACT.
- 7. REFER TO UTILITIES AND STRUCTURES MONITORING PLAN AND UTILITIES TIE-IN PLAN FOR RELOCATION OF EACH UTILITY AND FOR PROTECTION OF UTILITIES AND OTHER FACILITIES IN PLACE.

<u>GENERAL NOTES — TRAFFIC MANAGEMENT PLAN</u>

STRUCTURAL

Professional Seal

- 1. THE CONTRACTOR SHALL PREPARE A TRAFFIC MANAGEMENT PLAN (TMP) FOR THE PROJECT OR AMEND THE EXISTING TMP PREPARED FOR THE PROJECT
- 2. THE CONTRACTOR SHALL SUBMIT THE TMP TO THE CORPORATION, ENGINEER, TRANSLINK, AND TO THE CITY OF BURNABY FOR REVIEW AND APPROVAL PRIOR TO COMMENCING CONSTRUCTION

CIVIL

Professional Seal

GEOTECHNICAL

Professional Seal

- 3. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING THE CITY OF BURNABY AND TRANSLINK APPROVAL OF THE TMP PLAN, OR AMENDMENT TO THE EXISTING TMP.
- 4. A COPY OF THE EXISTING TRAFFIC MANAGEMENT GUIDELINES IS INCLUDED IN THE CONTRACT DOCUMENT'S ATTACHMENTS.

<u>GENERAL NOTES - SURVEY CONTROL</u>

- 1. COORDINATES ARE UTM NAD83(CSRS) 4.0.0.BC.1.GVRD CONVERTED TO GROUND-LEVEL USING A COMBINED SCALE FACTOR OF 0.999568802.
- 2. ELEVATIONS ARE IN METRES AND REFER TO CVD28GVRD2018 GEODETIC DATUM AND DERIVED FROM TABLET MARKING MON78H6975 — ELEV. 74.782m.
- 3. FIELD BOOK REF. #MS100 P29-40 AND #BD10 PG1. DATE OF SURVEY: AUGUST 12 TO AUGUST 18 AND OCTOBER 13, 2021. SEE DRAWING XC009WL192.DWG PROVIDED BY METRO VANCOUVER

							GREATER VANCOUVER SEWERAGE AND DRAINAGE DISTRICT			
							Design: MY	RETAINING WALL AT 2751 PRODUCTION WAY	SCALE: NTS	
P3	300CT24	GH	JT	AM	VS	ISSUED FOR PERMIT	Drawn: <u>JT</u>	BURNABY, BC	1113	
P2	24MAY24	GH	JT	AM	VS	100% DETAILED DESIGN SUBMISSION			DISTRICT FILE	
P1	19MAR24	GH	JT	AM	VS	90% DETAILED DESIGN SUBMISSION; DISTANCE FROM	Checked: AM	<u>. </u>	W-XXXX	
						RETAINING WALL TO BUILDING REDUCED FROM 9m TO 7.5m;	VS Approved		SHEET 2 OF 8	
		·				ADDED PROPOSED BC HYDRO DUCT BANK BEHIND RETAINING WALL	SG	GENERAL NOTES	DRAWING NUMBER	
Issue	Date	Desn	Dr'n	Chkd	Appd	Description	Manager			

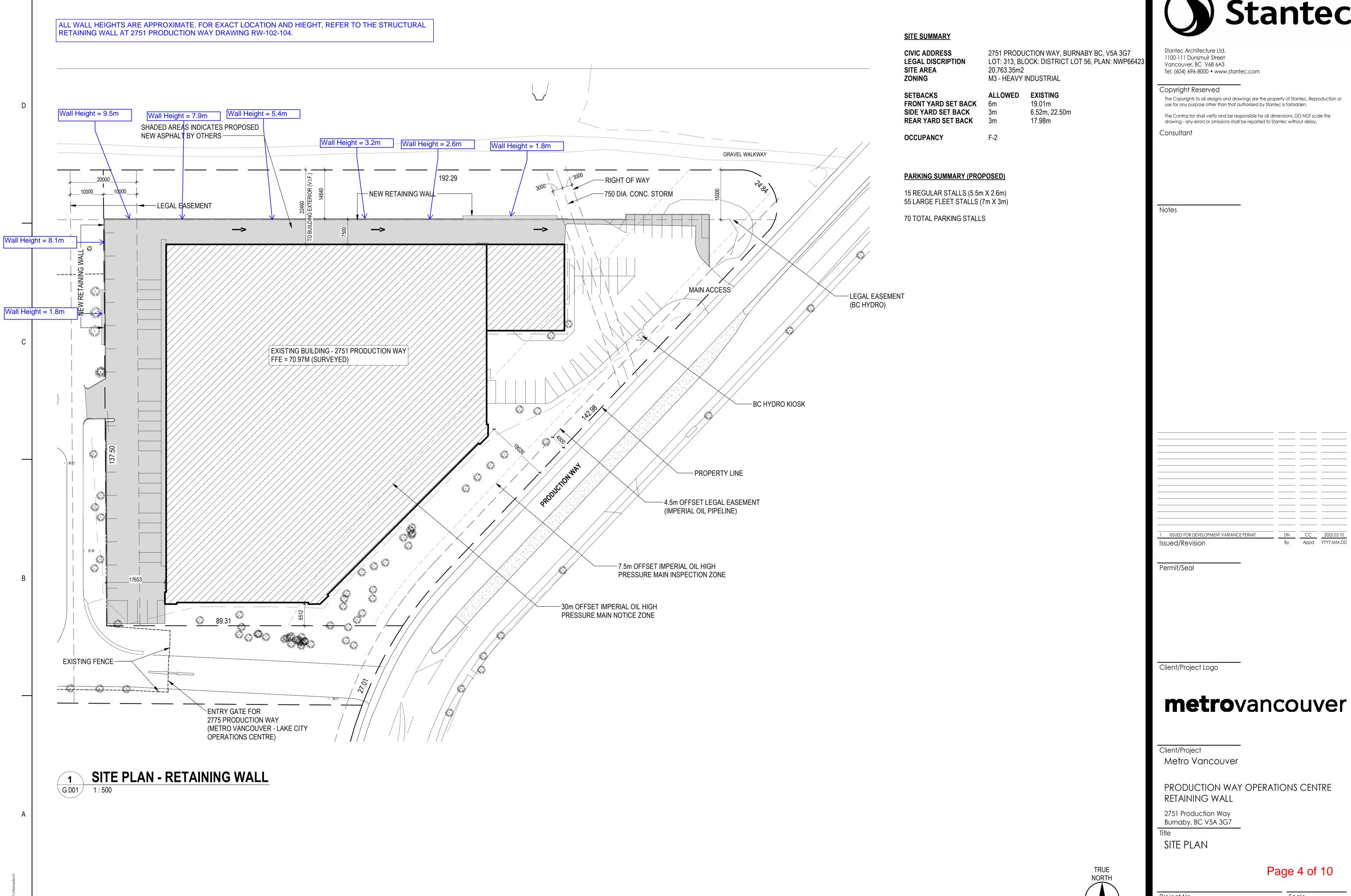
Bar is 20mm On Original Drawing. If Not On This Sheet, Adjust Scales Accordingly. | SUPERSEDES PRINTS OF THIS DRAWING NUMBER WITH LETTERS PREVIOUS TO 🖚 | P

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Client/Project Metro Vancouver

PRODUCTION WAY OPERATIONS CENTRE RETAINING WALL

2751 Production Way Burnaby, BC V5A 3G7

SITE PLAN

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Project No. Scale 144321073 As indicated Revision Drawing No.

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